

Koch's Soil Testing

P.O. Box 81
Loretto, MN 55357
Tel: (612) 479-2637

9/7/95

Bruce Kelly
2340 Fox St.
Orono, Minn.
Job Site; Above Address

Two sites were tested for the East lot (2340²⁶ Fox St.), the Site # 1 was tested on 5/26/95 and it was determine that a mound system would be needed due to the mottled soil levels at 24-28 inches (ave. perc. rate--25.6 min/in. While the Site # 2 area was tested on 8/4/95 and it was also determine that a mound system would be needed due to the mottled soil levels at 18-35 inches. The ave. perc. rate in this area was 35 min/in.

* Mottled soil is consider the high seasonal saturated soil conditions that exist during the wet seasons(fall and spring). The City of Orono Codes require that the minnimum depth to mottled soil be at 48 inches or deepeer before a standard trench system can be installed. Therefore with the above information the only type of system that can be installed on this lot and still comply with the local codes is a pressurized mound system.

In Late August I met with Mr. Steve Weckman (OronoCity Inspector) to look at the proposed mound system that were design in the 8/4/95 report.

After carefully determining the elevation of the propose mound system (mainly the Site # 2) it was determine that the Sites # 1 and # 2 mound system could be installed on this lot and still comply with the local Orono City Codes.

These proposed corner elevations of the proposed mound system were approved by Mr. Steve Weckman on the site in late August 1995.

Site Evaluator
Robert A Koch M.P.C.A. # 1429



*This report should be included in the Bruce Kelly Report dated 8/4/95 and the one dated 5/26/95.

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Bruce Kelly
2340 Fox St.
Orono, Minn.

8/4/95

Job Site ; Two lots at the above address

Two sites were tested to determine what type of drainfield could be installed and still comply with the local and the Minn. State Codes.

The west lot In the Site 1A area had an average percolation rate of 42.5 min/in. and the depth to mottled soil was at 23-24 inches. While in the Site-2A the ave. perc. rate was 39.2 min/in. and the depth mottled soil was at 24-28 inches.

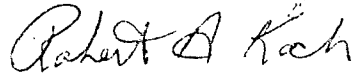
The results of the east 1/2 lot indicated that the Site #1 (tested on 5/26/95) had an ave.perc. rate of 25.6 min/in. and the depth to mottled soil was at 24-28 inches.

While in the Site # -2 (tested 7/25/95) had a average perc. rate of 35 min/in. and the depth to mottled soil was at 18-29 inches.

* Mottled soil is considered the seasonal saturated soil level that exist during the wet seasons. The City of Orono Codes state that the minimum depth to mottled soil be at 48 inches or deeper before a standard trench drainfield can be installed.

Therefore with the above information the only type of system that can be installed on these two lots is a pressurized mound system.

Site Evaluator



Robert A. Koch M.P.C.A# 1429

Koch's Soil Testing

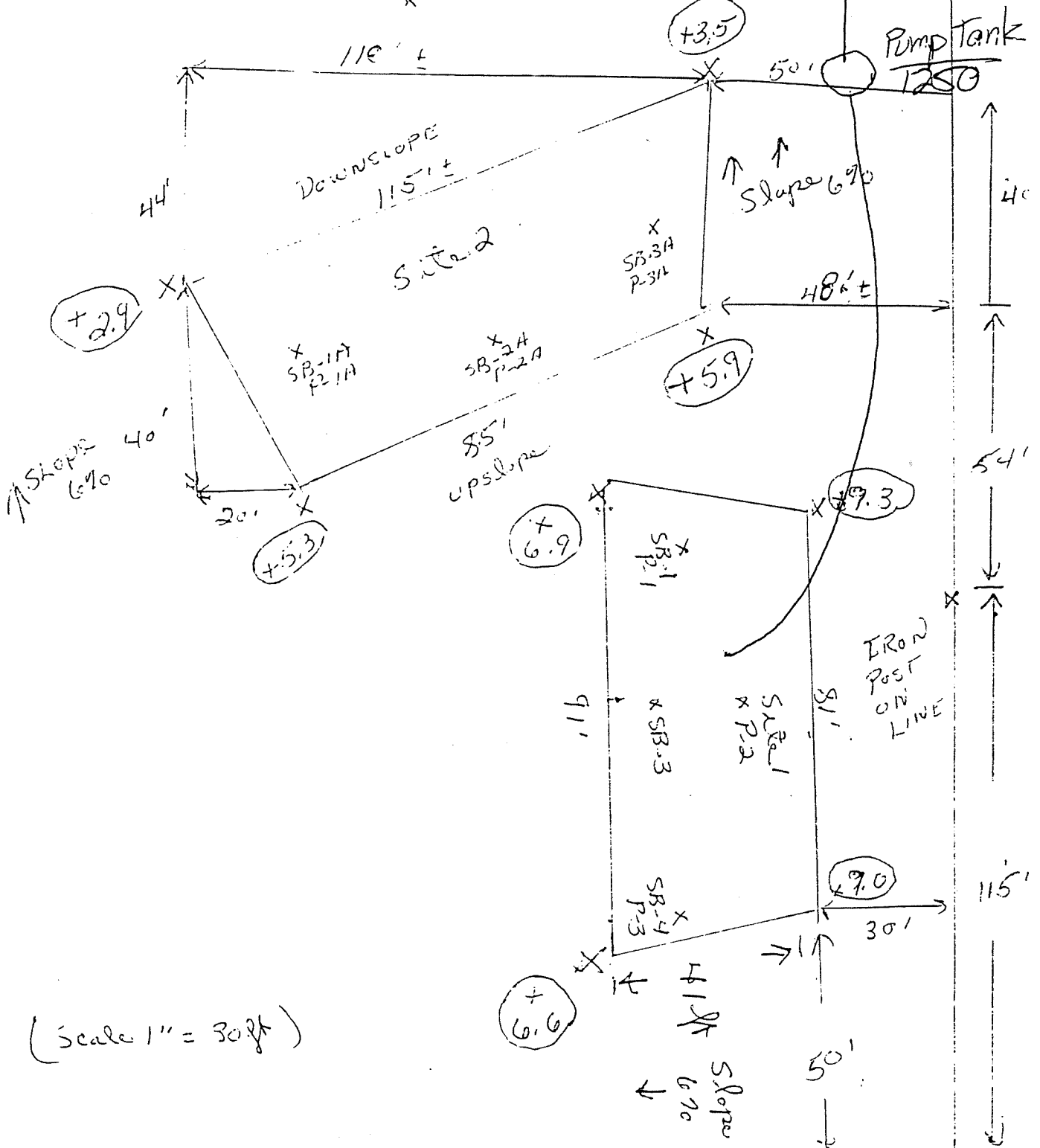
P.O. Box 81
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PROPOSE
HOUSE

N A

T.B.M. - ELEV. 0.00

X



(Scale 1" = 30ft)

LOT 5 T.

LOT PIN

MOUND DESIGN WORKSHEET (For Flows up to 1200 gpd)

Site 1

Tested 5/14/95

A. FLOW

Estimated 750 gpd (15 Bedroom)
or measured _____ x 1.5 = _____ gpd.

B. SEPTIC TANK LIQUID VOLUMES

2-1500 gallons

C. SOILS (refer to site evaluation)

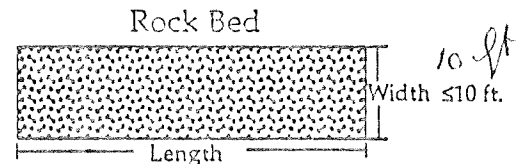
1. Depth to restricting layer = 24-29 inches
2. Depth of percolation tests = 12 inches
3. Percolation rate 25.6 mpi
4. Land slope 8.0 %

Estimated Sewage Flows in Gallons per day (gpd)				
Number of Bedrooms	Type I	Type II	Type III	Type IV
2	300	225	180	60% of the value in Type I, II or III columns
3	450	300	218	
4	600	375	256	
5	750	450	294	
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

Septic Tank Capacities, in gallons		
Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with sewage disposal
2 or less	750	1125
3 or 4	1000	1500
5 or 6	1500	2250
7, 8 or 9 over 9	2000	3000

D. ROCK LAYER DIMENSIONS

1. Multiply flow rate by 0.83 to obtain required area of rock layer: $A \times 0.83 =$
750 gpd $\times 0.83$ sq. ft./gpd = 630 sq. ft.
2. Select width of rock layer (10 feet or less) = 10 ft.
3. Length of rock layer = area $\div$ width =
630 sq. ft. $\div$ 10 ft. = 63 ft.



E. ROCK VOLUME

1. Multiply rock area by rock depth to get cubic feet of rock;
630 sq. ft. $\times$ 1 ft. = 630 cu. ft.
2. Divide cu. ft. by 27 cu. ft./cu. yd. to get cubic yards;
630 cu. ft. $\div 27 =$ 23 cu. yd.
3. Multiply cubic yards by 1.4 to get weight of rock in tons;
23 cu. yd. $\times 1.4$ ton/cu. yd. = 32 tons.

F. ADSORPTION WIDTH

1. Percolation rate in top 12 inches of soil is 25.6 mpi
Use 16-30 mpi Range (Loam)
2. Select allowable soil loading rate from table;
0.60 gpd/ft²
3. Calculate adsorption width ratio by dividing rock layer loading rate of 1.20 gpd/ft² by allowable soil loading rate;
 $1.20 \text{ gpd/ft}^2 \div 0.60 \text{ gpd/ft}^2 =$ 2.00
4. Multiply adsorption width ratio by rock layer width to get required adsorption width;
2.00 $\times$ 10 ft = 20 ft

Adsorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Gallons per day per square foot	Ratio of Adsorption width to Rock Layer Width
Faster than 0.1 *	Coarse Sand	---	---
0.1 to 5	Sand	1.20	1.00
0.1 to 5 **	Fine Sand	0.60	2.00
6 to 15	Sandy Loam	0.79	1.52
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam	0.50	2.40
46 to 60	Clay Loam	0.45	2.67
60 to 120	Clay	0.24	5.00
Slower than 120 ***	Clay	---	---

- * Soil too coarse for installation of a standard system.
See 7080.0170 Subp 2.G. 3, page 26.
- ** Soil having 50% or more of fine sand plus very fine sand.
- *** Soil too heavy for installation of a standard system.
See 7080.0210 Subp 5.A, page 33.

Sizing of Pump Station

1. Determine Surface Area

Rectangle = Area = L x W

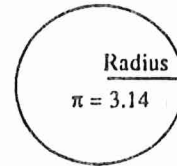
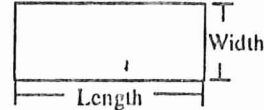
_____ x _____ = _____ square feet

Circle = Area = $\pi \times (\text{Radius})^2$

3.14 x _____ x _____ = _____ square feet

Other = Get Surface Area from Manufacturer

_____ square feet



2. Calculate Gallons Per Inch

There are 7.5 gallons per cubic foot of volume, therefore you must multiply the area times the conversion factor and divide by 12 inches per foot to calculate gallons per inch

Area x 7.5 gpd ÷ 12 inches per foot

1250 gal pump tank

_____ x 7.5 ÷ 12 = _____ gallons/inch

3. Calculate Gallons to Cover Pump (with 2 inches of water covering pump)

(Height (in) + 2 inches) x gallons/inch (#2)

(10 + 2) x 25 = 300 gallons

+ 2" Block 350

4. Calculate Total Pumpout Volume

a. To maximize pump life select pump size for 4 to 5 pump operations per day.

750 gpd ÷ 4 = 188 gallons per dose

b. Calculate drainback

1. Determine total pipe length, 100 feet.

2. Determine liquid volume of pipe, 17.4 gallons per 100 feet.

3. Multiply length by volume: Drainback quantity =

100 feet x 17.4 gallons/100 ft. = 17.4 gallons.

c. Total pump out volume equals dose volume + drainback

188 gallons per dose + 17.4 gallons = 205 gallons

5. Calculate Volume for Alarm (typically 2 to 3 inches)

Depth (in) x gallons/inch (#2) =

3 x 25 = 75 gallons

6. Calculate Reserve Capacity (75% the daily flow)

Daily flow (see page D-7) x .75 =

750 x .75 = _____ gallons

7. Calculate total gallons

gallons over pump + gallons pumpout + gallons alarm + gallons reserve capacity

#3 + #4 c + #5 + #6

300 + 205 + 75 + 563 = 1143 gallons

USE 1250 gal Minn. size

8. Total Depth (Total gallon divided by gallon per inch)

Total Gallon (#7) ÷ gallon/inch (#2)

1250 ÷ 25 = 50 inches

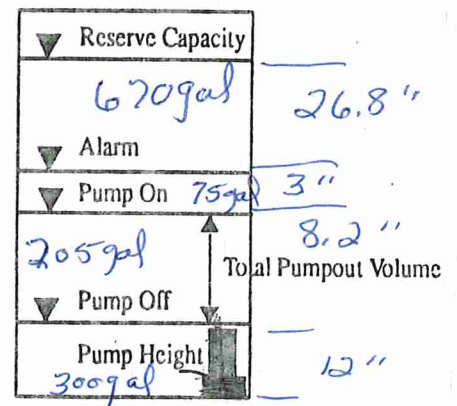
9. Float Separation Distance (equal total pumpout volume)

Total pumpout volume (#4c) ÷ gallons/inch (#2)

205 ÷ 25 = 8.2 inches

Estimated Sewage Flows in Gallons per day (gpd)				
Number of Bedrooms	Type I	Type II	Type III	Type IV
2	300	225	180	60% of the values in Type I, II or III columns
3	450	300	218	
4	600	375	256	
<u>5</u>	<u>750</u>	450	294	
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

Pipe diameter (inches)	Gallons per 100 feet
1	4.49
1.25	7.77
1.5	10.58
<u>2</u>	<u>17.43</u>
2.5	24.87
3	38.4
4	66.1



Site #1 (EAST LOT.)
2340 FOX ST.

PUMP SELECTION PROCEDURE

A. Determine pump capacity:

Gravity Distribution

- Minimum suggested is 600 gallons per hour (10 gpm) to stay ahead of water use rate.
- Maximum suggested for delivery to a drop box of a home system is 2,700 gallons per hour (45 gpm) to prevent build-up of pressure in drop box.

Pressure Distribution

- Select number of perforated laterals 3
- Select perforation spacing = 3 feet.
- Subtract 2 ft. from the rock layer length.
 $\frac{63}{\text{Rock layer length}} - 2 \text{ ft.} = \underline{61} \text{ feet.}$
- Determine the number of spaces between perforations.
Length perf. spacing = 61 ft. + 3 ft. = 20 spaces
- 20 spaces + 1 = 21 perforations/lateral
- Multiply perforations per lateral by number of laterals to get total number of perforations. $\frac{3}{\text{lateral}} \times \frac{21}{\text{perforations/lateral}} = \underline{63}$ perforations.
- $\frac{63}{\text{perforations}} \times \frac{0.74}{\text{gpm/perforation}} = \underline{47} \text{ gpm.} = \underline{50} \text{ gpm}$

SELECTED PUMP CAPACITY 50 gpm

Determine head requirements:

- Elevation difference between pump and point of discharge.
13 feet
- If pumping to a pressure distribution system, five feet for pressure required at manifold if gravity system, zero.
5 feet
- Friction loss
 - Enter friction loss table with gpm and pipe diameter. Read friction loss in feet per 100 feet from table.
F.L. = 3.99 ft./100 ft of pipe
 - Determine total pipe length from pump to discharge point. Add 25 percent to pipe length for fitting loss, or use a fitting loss chart. Equivalent pipe length = 1.25 times pipe length =
 $\frac{100}{\text{Pipe Length}} \times 1.25 = \underline{125}$ feet

Calculate total friction loss by multiplying friction loss in ft/100 ft by equivalent pipe length.

Total friction loss = $\underline{125} \times \underline{3.99} + 100 = \underline{5}$ feet

- Total head required is the sum of elevation difference, special head requirements, and total friction loss.

$$\frac{13}{(1)} + \frac{5}{(2)} + \frac{5}{(3c)}$$

TOTAL HEAD 23 feet

Pump selection

1. A pump must be selected to deliver at least 50 gpm (Step A) with at least 23 feet of total head (Step B).

END PERFORATION OF A PERFORATED LATERAL

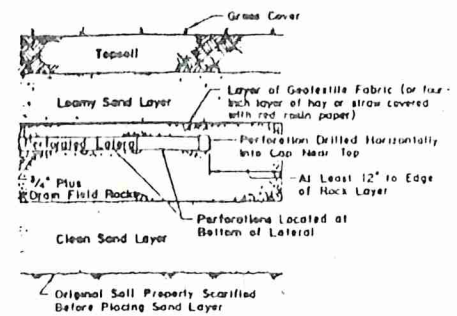
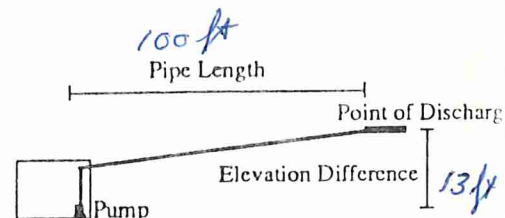


TABLE OF PERFORATION DISCHARGES IN GPM

Head	Perforation diameter (inches)	
	7/32	1/4
1.0a	0.56	0.74
1.5	0.69	0.90
2.0b	0.80	1.04
2.5	0.89	1.17
3.0	0.98	1.28
4.0	1.13	1.47
5.0	1.26	1.65

a Use 1.0 foot of head for residential systems.
b Use 2.0 feet of head for other establishments



F-18b

gpm	1.5 inch	2.0 inch	3.0 inch
	Friction loss per 100 ft of pipe		
10	0.69	0.20	
12	0.96	0.28	
14	1.28	0.38	
16	1.63	0.48	
18	2.03	0.60	
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	7.90	2.06	0.30
40	11.07	2.64	0.39
45	14.73	3.28	0.48
50		3.99	0.58
55		4.76	0.70
60		5.60	0.82

(Site 1)

G. DOWNSLOPE DIKE WIDTH

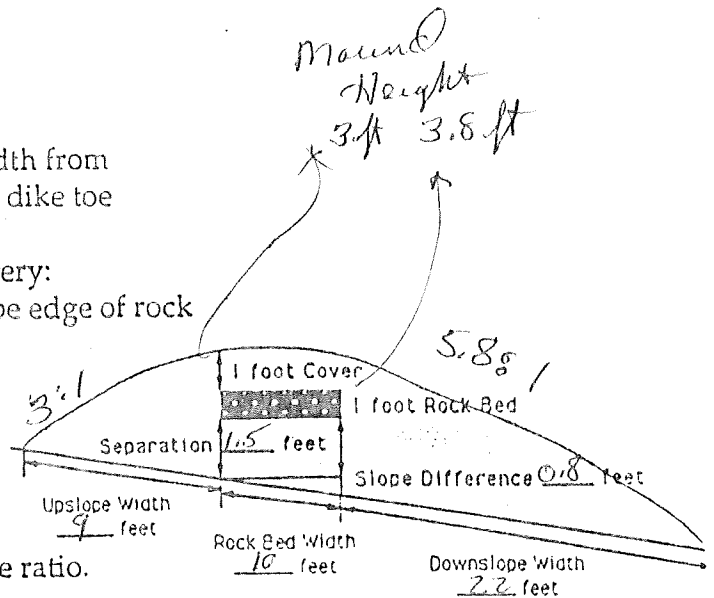
1. If landslope is 3% or more, subtract rock layer width from adsorption width to obtain minimum downslope dike toe

$$20 \text{ ft} - 10 \text{ ft} = 10 \text{ feet}$$
2. Calculate Minimum mound size based on geometry:
 - a. Determine depth of clean sand fill at upslope edge of rock layer: Separation : 3 ft - 2 = 1 feet
 - b. Add depth of clean sand for separation (2a) at upslope edge, depth of rock layer (1 foot) to depth of cover (1 foot) to find the mound height at the upslope edge of rock

$$1 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 3.0 \text{ feet}$$
 - c. Enter table with landslope and upslope dike ratio. Select dike multiplier of 3.00.
 - d. Multiply dike multiplier by upslope mound height to find upslope dike width: $3.00 \times 3.0 = 9$ feet
 - e. Multiply rock layer width by landslope to determine drop in elevation; Slope Difference

$$10 \times 8\% + 100 = 0.8 \text{ feet}$$
 - f. Add depth of clean sand for slope difference (2e) at downslope edge, to the mound height at the upslope edge of rock layer (2b) to find the downslope height;

$$3.0 \text{ ft} + 0.8 \text{ ft} = 3.8 \text{ feet}$$
 - g. Enter table with landslope and downslope dike ratio. Select dike multiplier of 5.88.
 - h. Multiply dike multiplier by downslope mound height (2f) to get downslope dike width: $3.8 \times 5.88 = 22$ feet
 - i. Compare the values of step G.1 and Step G.2h. Select the greater of the two values as the downslope dike width;
22 feet

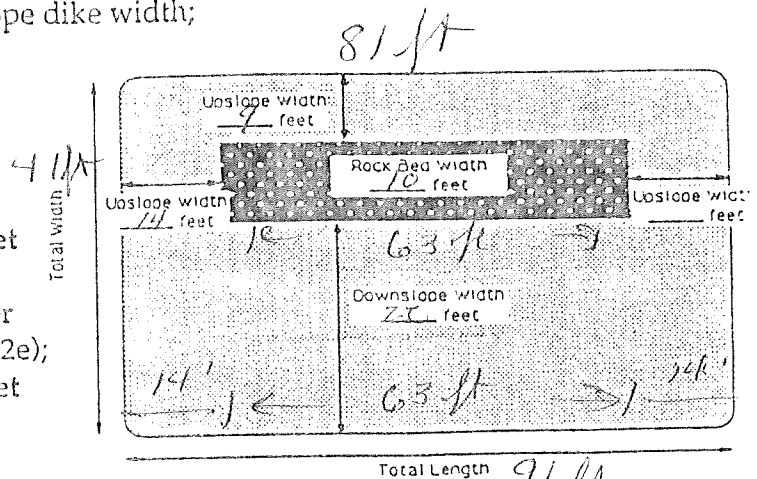


- j. Total mound width is the sum of upslope dike (G.2e) width plus rock layer width (D.2) plus downslope dike width (G.2i);

$$9 \text{ ft} + 10 \text{ ft} + 22 \text{ ft} = 41 \text{ feet}$$
- k. Total mound length is the sum of upslope dike width (G.2e) plus rock layer length (D.3) plus upslope dike width (G.2e);

$$9 \text{ ft} + 6.3 \text{ ft} + 9 \text{ ft} = 24.3 \text{ feet}$$

$$14.1 \text{ ft} + 6.3 \text{ ft} + 14.1 \text{ ft} = 34.5 \text{ ft}$$



% slope	Downslope					Upslope				
	3:1	4:1	5:1	6:1	7:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	3.0	4.0	5.0	6.0	7.0
1	3.09	4.17	5.26	6.38	7.53	2.91	3.85	4.76	5.66	6.54
2	3.19	4.35	5.56	6.82	8.14	2.83	3.70	4.54	5.36	6.16
3	3.30	4.54	5.88	7.32	8.86	2.75	3.57	4.35	5.03	5.79
4	3.41	4.76	6.25	7.89	9.72	2.68	3.45	4.17	4.84	5.46
5	3.53	5.00	6.67	8.57	10.77	2.61	3.33	4.00	4.62	5.19
6	3.66	5.26	7.14	9.38	12.07	2.54	3.23	3.85	4.41	4.93
7	3.80	5.56	7.69	10.34	13.73	2.48	3.12	3.70	4.23	4.70
8	3.95	5.88	8.33	11.54	15.91	2.42	3.03	3.57	4.05	4.49
9	4.11	6.25	9.09	13.04	18.92	2.36	2.94	3.45	3.90	4.30
10	4.29	6.67	10.0	15.00	23.33	2.31	2.86	3.33	3.75	4.12
11	4.48	7.14	11.11	17.65	30.43	2.26	2.78	3.23	3.61	3.95
12	4.69	7.69	12.50	21.43	43.75	2.21	2.70	3.12	3.49	3.80

Location or Project 2340 Fox (Proposed House)

Boring made by R. Kach

Date 5/4/95

Classification System: ASHTO ; USDA-SC X ; Unified ; Other

Auger used (check two): Hand X or Power ; Pilpht ; or Bucket X ; Other

Depth, in feet	Boring number <u>1</u> Surface elevation <u>94.4960</u> <u>Site 1</u>
0	<u>Topsail</u>
1	<u>Brown</u> <u>Black loam</u>
2	<u>10"</u>
3	<u>Brown clay</u>
4	
5	<u>60"</u>
6	
7	
8	
9	
10	

End of boring at 5 feet.

Standing water table:

Present at _____ feet of depth,
_____ hours after boring.

Not present in boring hole X.

Mottled soil:

Observed at 26" feet of depth.

Not present in boring hole _____.

Depth, in feet	Boring number <u>2</u> Surface elevation <u>93.8</u> <u>Site 1</u>
0	<u>Topsail</u>
1	<u>Brown loam</u>
2	<u>7"</u>
3	<u>Brown clay</u>
4	<u>13"</u>
5	<u>BRDWN SANDY CLAY</u>
6	<u>22"</u>
7	<u>Brown clay</u>
8	<u>60"</u>
9	
10	

End of boring at 5 feet.

Standing water table:

Present at _____ feet of depth,
_____ hours after boring.

Not present in boring hole X.

Mottled soil:

Observed at 24" feet of depth.

Not present in boring hole _____.

Location or Project 2340 Fox St (Proposed House)

Boring made by R. Kew

Date 5/4/95

Classification System: ASHTO ; USDA-SCS X ; Unified ; other

Auger used (check two): Hand X or Power ; Flight ; or Bucket X other

Depth, in feet	Boring number <u>3</u> Surface elevation <u>94.6</u> <u>Site 1</u>
0	Topsail
1	Brown CLAY loam
2	9" <u> </u> Brown CLAY
3	32" <u> </u>
4	Brown SANDY CLAY
5	38" <u> </u>
6	Brown CLAY
7	60" <u> </u>
8	
9	
10	

End of boring at 5 feet.

Standing water table

Present at feet of depth,
 hours after boring.

Not present in boring hole X.

Mottled soils

Observed at 24" feet of depth.

Not present in boring hole .

Depth, in feet	Boring number <u>4</u> Surface elevation <u>93.6</u> <u>Site 1</u>
0	Topsail
1	Brown CLAY loam
2	12" <u> </u> Brown CLAY
3	48" <u> </u>
4	Brown SANDY CLAY
5	60" <u> </u>
6	
7	
8	
9	
10	

End of boring at 5 feet.

Standing water table

Present at feet of depth,
 hours after boring.

Not present in boring hole X.

Mottled soils

Observed at 28" feet of depth.

Not present in boring hole .

Proposed Name

- PERCOLATION TEST SHEET -

Site 1

Test hole location 2340 Fox ST

Hole # 1

Date test hole was prepared: 5/4/95

Depth of hole bottom: 12 inches

Diameter of hole: 6 inches

Soil Data from test hole:

depth, inches

0 - 10

10 - 12

soil texture:

Loam Topsoil

CLAY

soil color

Brown

BROWN

Method of scratching sidewall: Nail

Depth of pea size gravel in bottom of hole: 2 inches

Date and hour of initial water filling: 5/4/95 1pm Depth of initial water filling: 12 above hole bottom

Method used to maintain 12" of water depth in hole for 4 hours: Autometer Siphon

Percolation test conducted by: R. Ken Percolation test started at 10 (am) pm.

Maximum water depth above hole bottom during test: 6 inches

5/5/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	conversions
<u>10 -</u> <u>10:20</u>	START <u>20</u>	<u>6</u> <u>5</u>	<u>1</u>	<u>1</u>	$\frac{20}{\text{TIME}} \div \frac{1}{\text{DROP}} = \frac{20.0}{\text{PERC}} A$	1/16 = .06 1/8 = .13 3/16 = .19
<u>10:21</u> <u>10:41</u>	REFILL <u>20</u>	<u>6</u> <u>5 11/16</u>	<u>15/16</u>	<u>0.94</u>	$\frac{20}{\text{TIME}} \div \frac{0.94}{\text{DROP}} = \frac{21.3}{\text{PERC}} B$	1/4 = .25 5/16 = .31
<u>10:42</u> <u>11:02</u>	REFILL <u>20</u>	<u>6</u> <u>5 7/8</u>	<u>1/8</u>	<u>0.88</u>	$\frac{20}{\text{TIME}} \div \frac{0.88}{\text{DROP}} = \frac{22.7}{\text{PERC}} C$	3/8 = .38 7/16 = .44
<u>11:03</u> <u>11:23</u>	REFILL <u>20</u>	<u>6</u> <u>5 7/8</u>	<u>1/8</u>	<u>0.88</u>	$\frac{20}{\text{TIME}} \div \frac{0.88}{\text{DROP}} = \frac{22.7}{\text{PERC}} D$	1/2 = .5 9/16 = .56
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{PERC}} E$	5/8 = .63
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{PERC}} F$	11/16 = .69
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{PERC}} G$	3/4 = .75 13/16 = .81
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{PERC}} H$	7/8 = .88 15/16 = .94

Ten Percent Calculation *

A, B, C
 $\frac{22.7}{\text{Largest \# of ABC}} - \frac{20}{\text{Smallest \# of ABC}} = \frac{2.7}{\text{Average \# of ABC}}$
 $\frac{20}{\text{Smallest \# of ABC}} \times 0.10 = \frac{2.0}{\text{Average \# of ABC}}$

C, D, E
 $\frac{\text{Largest \# of CDE}}{\text{Smallest \# of CDE}} = \text{Average \# of CDE}$
 $\frac{\text{Smallest \# of CDE}}{\text{Smallest \# of CDE}} \times 0.10 = \text{Average \# of CDE}$

E, F, G
 $\frac{\text{Largest \# of EFG}}{\text{Smallest \# of EFG}} = \text{Average \# of EFG}$
 $\frac{\text{Smallest \# of EFG}}{\text{Smallest \# of EFG}} \times 0.10 = \text{Average \# of EFG}$

B, C, D
 $\frac{22.7}{\text{Largest \# of BCD}} - \frac{21.3}{\text{Smallest \# of BCD}} = \frac{1.4}{\text{Average \# of BCD}}$
 $\frac{21.3}{\text{Smallest \# of BCD}} \times 0.10 = \frac{2.1}{\text{Average \# of BCD}}$

D, E, F
 $\frac{22.7}{\text{Largest \# of DEF}} - \frac{21.3}{\text{Smallest \# of DEF}} = \frac{22.2}{\text{Average \# of DEF}}$
 $\frac{21.3}{\text{Smallest \# of DEF}} \times 0.10 = \frac{2.1}{\text{Average \# of DEF}}$

F, G, H
 $\frac{\text{Largest \# of FGH}}{\text{Smallest \# of FGH}} = \text{Average \# of FGH}$
 $\frac{\text{Smallest \# of FGH}}{\text{Smallest \# of FGH}} \times 0.10 = \text{Average \# of FGH}$

* If the top number in each set of boxes is larger than the bottom number then take another reading. If the top number is equal or smaller than bottom number, average the three numbers for the perc rate.

PERCOLATION TEST SHEET - Site 1

Test hole location 2340 Fox Hole # 2 Date test hole was prepared: 5/4/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches
0-10"
10-12
 soil texture: Loam Topsoil
CLAY
 soil color: Brown
Brown

Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 5/4/95 1pm Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: Automatic Siphon
 Percolation test conducted by: R. Koch Percolation test started at: 10 (am) pm.
 Maximum water depth above hole bottom during test: 6 inches 5/5/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	CONVERSIONS
<u>1003</u> <u>1023</u>	START <u>20</u>	<u>6</u> <u>5 1/8</u>	<u>7/8</u>	<u>0.88</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{A}}$ $\frac{20}{20} \div \frac{0.88}{\text{A}} = \frac{22.7}{\text{A}}$	1/16 = .06 1/8 = .13 3/16 = .19
<u>1026</u> <u>1046</u>	REFILL <u>20</u>	<u>6</u> <u>5 3/8</u>	<u>13/16</u>	<u>0.81</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{B}}$ $\frac{20}{20} \div \frac{0.81}{\text{B}} = \frac{24.7}{\text{B}}$	1/4 = .25 5/16 = .31
<u>1048</u> <u>1108</u>	REFILL <u>20</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{C}}$ $\frac{20}{20} \div \frac{0.75}{\text{C}} = \frac{26.7}{\text{C}}$	3/8 = .38 7/16 = .44
<u>1110</u> <u>1130</u>	REFILL <u>20</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{D}}$ $\frac{20}{20} \div \frac{0.75}{\text{D}} = \frac{26.7}{\text{D}}$	1/2 = .5 9/16 = .56
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{---}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{E}}$	5/8 = .63
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{---}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{F}}$	11/16 = .69
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{---}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{G}}$	3/4 = .75 13/16 = .81
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{---}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{H}}$	7/8 = .88 15/16 = .94

Ten Percent Calculation *

A, B, C
 $\frac{26.7}{\text{Largest \# of ABC}} - \frac{22.7}{\text{Smallest \# of ABC}} = \frac{4.0}{\text{---}}$
 $\frac{22.7}{\text{Smallest \# of ABC}} \times 0.10 = \frac{2.7}{\text{---}}$

C, D, E
 $\frac{\text{---}}{\text{Largest \# of CDE}} - \frac{\text{---}}{\text{Smallest \# of CDE}} = \text{---}$
 $\frac{\text{---}}{\text{Smallest \# of CDE}} \times 0.10 = \text{---}$

E, F, G
 $\frac{\text{---}}{\text{Largest \# of EFG}} - \frac{\text{---}}{\text{Smallest \# of EFG}} = \text{---}$
 $\frac{\text{---}}{\text{Smallest \# of EFG}} \times 0.10 = \text{---}$

B, C, D
 $\frac{26.7}{\text{Largest \# of BCD}} - \frac{24.7}{\text{Smallest \# of BCD}} = \frac{2.0}{\text{---}}$
 $\frac{24.7}{\text{Smallest \# of BCD}} \times 0.10 = \frac{2.5}{\text{---}}$

D, E, F
 $\frac{\text{---}}{\text{Largest \# of DEF}} - \frac{\text{---}}{\text{Smallest \# of DEF}} = \text{---}$
 $\frac{\text{---}}{\text{Smallest \# of DEF}} \times 0.10 = \text{---}$

F, G, H
 $\frac{\text{---}}{\text{Largest \# of FGH}} - \frac{\text{---}}{\text{Smallest \# of FGH}} = \text{---}$
 $\frac{\text{---}}{\text{Smallest \# of FGH}} \times 0.10 = \text{---}$

* If the top number in each set of boxes is larger than the bottom number then take another reading. If the top number is equal or smaller than bottom number, average the three numbers for the perc rate.

Proposed Name - PERCOLATION TEST SHEET - Site 1

Test hole location 2340 Fox St Hole # 3 Date test hole was prepared: 5/4/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches <u>0-12"</u>	soil texture: <u>Loam Topsoil</u>	soil color <u>Brown</u>
_____	_____	_____
_____	_____	_____
_____	_____	_____

Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 5/4/95 6pm Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: Automatic Siphon
 Percolation test conducted by: R. Kach Percolation test started at 10 (am) pm.
 Maximum water depth above hole bottom during test: 12 inches 5/5/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	CONVERSIONS
<u>1006</u>	START	<u>6</u>				1/16 = .06
<u>1026</u>	<u>20</u>	<u>5 3/16</u>	<u>13/16</u>	<u>0.81</u>	$\frac{20}{\text{TIME}} \div \frac{0.81}{\text{DROP (Decimal)}} = \frac{24.7}{\text{PERC}} A$	1/8 = .13
<u>1028</u>	REFILL	<u>6</u>				3/16 = .19
<u>1048</u>	<u>20</u>	<u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{20}{\text{TIME}} \div \frac{0.75}{\text{DROP (Decimal)}} = \frac{26.7}{\text{PERC}} B$	1/4 = .25
<u>1050</u>	REFILL	<u>6</u>				5/16 = .31
<u>1110</u>	<u>20</u>	<u>5 3/16</u>	<u>11/16</u>	<u>0.69</u>	$\frac{20}{\text{TIME}} \div \frac{0.69}{\text{DROP (Decimal)}} = \frac{28.9}{\text{PERC}} C$	3/8 = .38
<u>1113</u>	REFILL	<u>6</u>				7/16 = .44
<u>1133</u>	<u>20</u>	<u>5 3/16</u>	<u>11/16</u>	<u>0.69</u>	$\frac{20}{\text{TIME}} \div \frac{0.69}{\text{DROP (Decimal)}} = \frac{28.9}{\text{PERC}} D$	1/2 = .5
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC} E$	9/16 = .56
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC} F$	5/8 = .63
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC} G$	11/16 = .69
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC} H$	3/4 = .75
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC}$	13/16 = .81
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC}$	7/8 = .88
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP (Decimal)}} = \text{PERC}$	15/16 = .94

Ten Percent Calculation *

A, B, C
$\frac{28.9}{\text{Largest \# of ABC}} - \frac{24.7}{\text{Smallest \# of ABC}} = 4.2$
$\frac{24.7}{\text{Smallest \# of ABC}} \times 0.10 = 2.5$
C, D, E
$\frac{\text{Largest \# of CDE}}{\text{Smallest \# of CDE}} = \text{_____}$
$\frac{\text{Smallest \# of CDE}}{\text{Smallest \# of CDE}} \times 0.10 = \text{_____}$
E, F, G
$\frac{\text{Largest \# of EFG}}{\text{Smallest \# of EFG}} = \text{_____}$
$\frac{\text{Smallest \# of EFG}}{\text{Smallest \# of EFG}} \times 0.10 = \text{_____}$

B, C, D
$\frac{28.9}{\text{Largest \# of BCD}} - \frac{24.7}{\text{Smallest \# of BCD}} = 2.2$
$\frac{26.7}{\text{Smallest \# of BCD}} \times 0.10 = 2.7$
D, E, F
$\frac{\text{Largest \# of DEF}}{\text{Smallest \# of DEF}} = \text{_____}$
$\frac{\text{Smallest \# of DEF}}{\text{Smallest \# of DEF}} \times 0.10 = 3$
F, G, H
$\frac{\text{Largest \# of FGH}}{\text{Smallest \# of FGH}} = \text{_____}$
$\frac{\text{Smallest \# of FGH}}{\text{Smallest \# of FGH}} \times 0.10 = \text{_____}$

If the top number in each set of boxes is larger than the bottom number then take another reading. If the top number is equal or smaller than bottom number, average the three numbers together.

PERCOLATION TEST SHEET - Site 1

Test hole location 2340 Fox St Hole # 4 Date test hole was prepared: 5/4/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches
0-7"
7-12
 soil texture:
Loam Topsoil
CLAY
 soil color
BROWN
BROWN

Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 5/4/95 1PM Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: Automatic Siphon
 Percolation test conducted by: R. Koch Percolation test started at 10:00 AM
 Maximum water depth above hole bottom during test: 6 inches 5/5/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	conversions
<u>1009</u> <u>1029</u>	START <u>20</u>	<u>6</u> <u>5 11/16</u>	<u>15/16</u>	<u>0.94</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{DECIMAL}} = \frac{21.3A}{\text{PERC}}$	1/16 = .06 1/8 = .13 3/16 = .19
<u>1037</u> <u>1052</u>	REFILL <u>20</u>	<u>6</u> <u>5 3/16</u>	<u>13/16</u>	<u>0.81</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{DECIMAL}} = \frac{24.6B}{\text{PERC}}$	1/4 = .25 5/16 = .31
<u>1053</u> <u>1113</u>	REFILL <u>20</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{DECIMAL}} = \frac{26.7C}{\text{PERC}}$	3/8 = .38 7/16 = .44
<u>1116</u> <u>1136</u>	REFILL <u>20</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{20}{\text{TIME}} \div \frac{\text{DROP}}{\text{DECIMAL}} = \frac{26.7D}{\text{PERC}}$	1/2 = .5 9/16 = .56
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{DECIMAL}} E$	5/8 = .63
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{DECIMAL}} F$	11/16 = .69
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{DECIMAL}} G$	3/4 = .75
—	REFILL	—	—	—	$\frac{\text{TIME}}{\text{DROP}} = \frac{\text{PERC}}{\text{DECIMAL}} H$	13/16 = .81 7/8 = .88 15/16 = .94

Ten Percent Calculation *

A, B, C	<u>26.7</u> - <u>21.3</u> = <u>5.4</u>
Largest # of ABC	Smallest # of ABC
<u>21.3</u>	$\times 0.10 = \underline{2.1}$
C, D, E	—
Largest # of CDE	Smallest # of CDE
—	$\times 0.10 = \underline{\quad}$
E, F, G	—
Largest # of EFG	Smallest # of EFG
—	$\times 0.10 = \underline{\quad}$

B, C, D	<u>26.7</u> - <u>24.6</u> = <u>2.1</u>
Largest # of BCD	Smallest # of BCD
<u>24.6</u>	$\times 0.10 = \underline{2.5}$
D, E, F	<u>26.7</u> - <u>26.7</u> = <u>0</u>
Largest # of DEF	Smallest # of DEF
—	$\times 0.10 = \underline{3}$
F, G, H	—
Largest # of FGH	Smallest # of FGH
—	$\times 0.10 = \underline{\quad}$

If the top number in each set of boxes is larger than the bottom number then take another reading. If the top number is equal or smaller than bottom

MOUND DESIGN WORKSHEET *Site 2* (For Flows up to 1200 gpd)

A. FLOW

Estimated 750 gpd (*5 Bedrooms*)
or measured _____ x 1.5 = _____ gpd.

B. SEPTIC TANK LIQUID VOLUMES

2-1500 gallons

C. SOILS (refer to site evaluation)

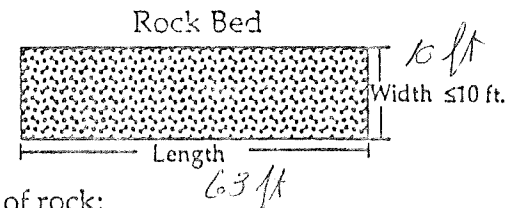
1. Depth to restricting layer = 18-29 inches
2. Depth of percolation tests = 12 inches
3. Percolation rate 35.6 mpi
4. Land slope 6.0 %

Estimated Sewage Flows in Gallons per day (gpd)				
Number of Bedrooms	Type I	Type II	Type III	Type IV
2	300	225	180	60% of the values in Type I, II, or III columns
3	450	300	218	
4	600	375	256	
5	750	450	294	
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

Septic Tank Capacities, in gallons		
Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal
2 or less	750	1125
3 or 4	1000	1500
5 or 6	1500	2250
7, 8 or 9 over 9	2000	3000

D. ROCK LAYER DIMENSIONS

1. Multiply flow rate by 0.83 to obtain required area of rock layer: $A \times 0.83 =$
750 gpd $\times 0.83$ sq. ft./gpd = 630 sq. ft.
2. Select width of rock layer (10 feet or less) = 10 ft.
3. Length of rock layer = area $\div$ width =
630 sq. ft. $\div$ 10 ft. = 63 ft.



E. ROCK VOLUME

1. Multiply rock area by rock depth to get cubic feet of rock;
630 sq. ft. $\times$ 1 ft. = 630 cu. ft.
2. Divide cu. ft. by 27 cu. ft./cu. yd. to get cubic yards;
630 cu. ft. $\div 27 =$ 24 cu. yd.
3. Multiply cubic yards by 1.4 to get weight of rock in tons;
24 cu. yd. $\times 1.4$ ton/cu. yd. = 34 tons.

F. ADSORPTION WIDTH

1. Percolation rate in top 12 inches of soil is 35 mpi
are 31-45 mpi Range
2. Select allowable soil loading rate from table;
0.50 gpd/ft²
3. Calculate adsorption width ratio by dividing rock layer loading rate of 1.20 gpd/ft² by allowable soil loading rate;
 $1.20 \text{ gpd/ft}^2 \div 0.50 \text{ gpd/ft}^2 =$ 2.4
4. Multiply adsorption width ratio by rock layer width to get required adsorption width;
2.4 $\times$ 10 ft = 24 ft

Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Gallons per day per square foot	Ratio of Adsorption width to Rock Layer Width
Faster than 0.1 *	Coarse Sand	---	---
0.1 to 5	Sand	1.20	1.00
0.1 to 5 **	Fine Sand **	0.60	2.00
6 to 15	Sandy Loam	0.79	1.52
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam	0.50	2.40
46 to 60	Clay Loam	0.45	2.67
60 to 120	Clay	0.24	5.00
Slower than 120 ***	Clay	---	---

- * Soil too coarse for installation of a standard system.
See 7080.0170 Subp 2.G. 3, page 26.
- ** Soil having 50% or more of fine sand plus very fine sand.
- *** Soil too heavy for installation of a standard system.
See 7080.0210 Subp 5.A, page 33.

East lot - Site 2

G. DOWNSLOPE DIKE WIDTH

1. If landslope is 3% or more, subtract rock layer width from adsorption width to obtain minimum downslope dike toe

$34 \text{ ft} - 10 \text{ ft} = 14 \text{ feet}$

2. Calculate Minimum mound size based on geometry:

a. Determine depth of clean sand fill at upslope edge of rock layer: Separation : $3 \text{ ft} - 1.5 = 1.5 \text{ feet}$

b. Add depth of clean sand for separation (2a) at upslope edge, depth of rock layer (1 foot) to depth of cover (1 foot) to find the mound height at the upslope edge of rock

$1.5 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 3.5 \text{ feet}$

c. Enter table with landslope and upslope dike ratio.

Select dike multiplier of 3.23

d. Multiply dike multiplier by upslope mound height to find upslope dike width: $3.5 \times 3.23 = 11 \text{ feet}$

e. Multiply rock layer width by landslopt to determine drop in elevation; Slope Difference

$10 \times 6 \% + 100 = 0.6 \text{ feet}$

f. Add depth of clean sand for slope difference (2e) at downslope edge, to the mound height at the upslope edge of rock layer (2b) to find the downslope height;

$3.5 \text{ ft} + 0.6 \text{ ft} = 4.1 \text{ feet}$

g. Enter table with landslope and downslope dike ratio.

Select dike multiplier of 5.26

h. Multiply dike multiplier by downslope mound height (2f) to get downslope dike width: $4.1 \times 5.26 = 22 \text{ feet}$

i. Compare the values of step G.1 and Step G.2h Select the greater of the two values as the downslope dike width;

22 feet

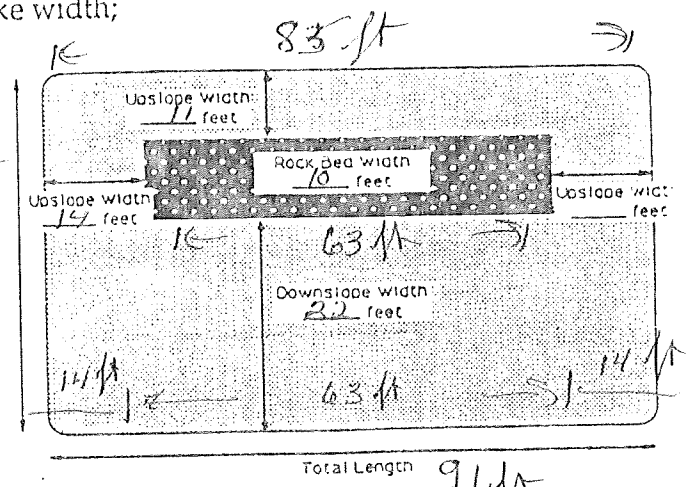
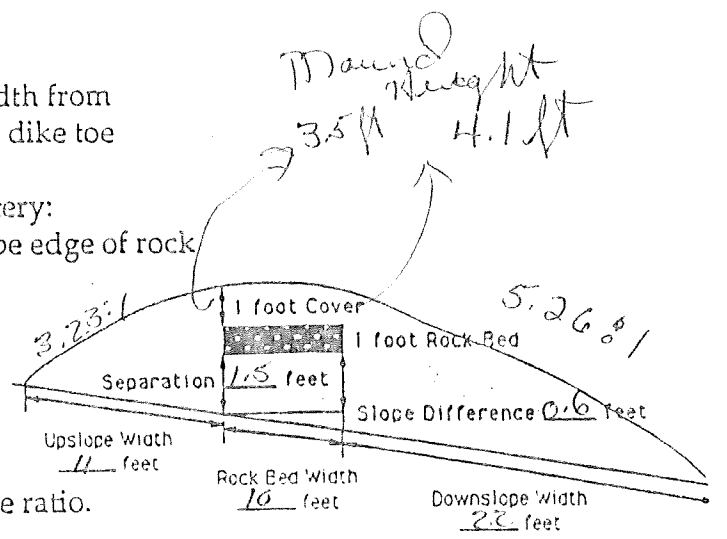
j. Total mound width is the sum of upslope dike (G.2e) width plus rock layer width (D.2) plus downslope dike width(G.2i);

$11 \text{ ft} + 10 \text{ ft} + 22 \text{ ft} = 43 \text{ feet}$

k. Total mound length is the sum of upslope dike width (G.2e) plus rock layer length (D.3) plus upslope dike width (G.2e);

$11 \text{ ft} + 63 \text{ ft} + 11 \text{ ft} = 85 \text{ feet}$

$14 \text{ ft} + 63 \text{ ft} + 14 \text{ ft} = 91 \text{ ft}$



% slope	Downslope					Upslope				
	3:1	4:1	5:1	6:1	7:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	3.0	4.0	5.0	6.0	7.0
1	3.09	4.17	5.25	6.33	7.41	2.91	3.85	4.76	5.66	6.54
2	3.19	4.35	5.56	6.82	8.14	2.83	3.70	4.54	5.36	6.14
3	3.30	4.54	5.88	7.32	8.86	2.75	3.57	4.35	5.08	5.79
4	3.41	4.76	6.25	7.89	9.72	2.68	3.45	4.17	4.84	5.46
5	3.53	5.00	6.67	8.57	10.77	2.61	3.33	4.00	4.62	5.19
6	3.66	5.26	7.14	9.38	12.07	2.54	3.23	3.85	4.41	4.93
7	3.80	5.56	7.69	10.34	13.73	2.48	3.12	3.70	4.23	4.70
8	3.95	5.88	8.33	11.54	15.91	2.42	3.03	3.57	4.05	4.49
9	4.11	6.25	9.09	13.04	18.92	2.36	2.94	3.45	3.90	4.30
10	4.29	6.67	10.0	15.00	23.33	2.31	2.86	3.33	3.75	4.12
11	4.48	7.14	11.11	17.65	30.43	2.26	2.78	3.23	3.61	3.95
12	4.69	7.69	12.50	21.43	43.75	2.21	2.70	3.12	3.49	3.80

Location or Project 2340 Fox ST (East Lot) ORANO Site 2
Borings made by R. Koch Date 7/25/95
Classification System: AASHTO ; USDA-SCS 1 Unfilled 1 other
Auger used (check two): Hand X or Power 1 Pilpht 1 or Bucket X other

Depth, in feet	Boring number <u>1A</u> Surface elevation _____ <u>Site 2</u>
0 —	
1 —	Topsoil Black loam
2 —	12" _____ Brown sandy clay
3 —	
4 —	
5 —	60" _____
6 —	
7 —	
8 —	
9 —	
10 —	

End of boring at 5 feet.

Standing water table:

Present at _____ feet of depth,

_____ hours after boring.

Not present in boring hole X.

Mottled soils:

Observed at 29" X of depth.

Not present in boring hole _____

Depth, in feet	Boring number <u>2A</u> Surface elevation _____ <u>Site 2</u>
0 —	
1 —	Topsoil Black loam
2 —	6" _____ Brown clay
3 —	
4 —	
5 —	60" _____
6 —	
7 —	
8 —	
9 —	
10 —	

End of boring at 5 feet.

Standing water table:

Present at _____ feet of depth,

_____ hours after boring.

Not present in boring hole X.

Mottled soils:

Observed at 18" X of depth.

Not present in boring hole _____

Location or Project 2340 Pot ST. (East Lot) Orowo Side 2
 Boring made by R. Koch Date 7/25/95
 Classification System: MSHO ; USDA-SC ☒ Unfilled ☐ other
 Auger used (check two): Hand ☒ or Power ☐ Pilpit ☐ or Bucket ☒ other ☐

Depth, in feet	Boring number <u>3A</u> Surface elevation _____ <u>Side 2</u>
0 —	
1 —	<u>Topsail</u>
	<u>Black Loam</u>
2 —	<u>18"</u>
3 —	<u>Brown clay</u>
4 —	<u>30"</u>
5 —	<u>Brown Sandy Clay</u>
6 —	<u>34"</u>
	<u>Brown clay</u>
7 —	<u>60"</u>
8 —	
9 —	
10 —	

End of boring at 5 feet.
 Standing water table:
 Present at _____ feet of depth,
 _____ hours after boring.
 Not present in boring hole ☒.

Rotated soils:
 Observed at 26" feet of depth.
 Not present in boring hole _____

Depth, in feet	Boring number _____ Surface elevation _____
0 —	
1 —	
2 —	
3 —	
4 —	
5 —	
6 —	
7 —	
8 —	
9 —	
10 —	

End of boring at _____ feet.
 Standing water table:
 Present at _____ feet of depth,
 _____ hours after boring.
 Not present in boring hole _____

Rotated soils:
 Observed at _____ feet of depth.
 Not present in boring hole _____

PERCOLATION TEST SHEET - Side 2

Test hole location 2346 Fox St (East) Hole # 1A Date test hole was prepared: 7/25/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches 0-12 soil texture: Loam Topsoil soil color: Black

Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 7/25/95 3pm Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: _____
 Percolation test conducted by: R. Korn Percolation test started at 1 (am/pm)
 Maximum water depth above hole bottom during test: 6 inches 7/26/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	
<u>1-130</u>	START <u>30</u>	<u>6</u> <u>5 1/16</u>	<u>15/16</u>	<u>0.94</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{31.9}{\text{PERC}} - A$	115 = 05
<u>132-202</u>	REFILL <u>30</u>	<u>6</u> <u>5 1/16</u>	<u>15/16</u>	<u>0.94</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{31.9}{\text{PERC}} - B$	118 = 13
<u>204-239</u>	REFILL <u>30</u>	<u>6</u> <u>5 1/4</u>	<u>7/8</u>	<u>0.88</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{34.1}{\text{PERC}} - C$	116 = 13
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - D$	114 = 25
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - E$	115 = 31
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - F$	113 = 53
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - G$	112 = 41
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - H$	110 = 3
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - I$	105 = 56
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - J$	53 = 63
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - K$	100 = 63
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - L$	112 = 75
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - M$	1116 = 21
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - N$	73 = 13
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{DROP}} \div \text{PERC} - O$	1016 = 54

Ten Percent Calculation *

A, B, C
 $\frac{31.9}{3} = 10.63$
 $\frac{31.9}{3} \times 0.10 = 3.2$
 D, E, F
 $\frac{31.9 + 31.9 + 34.1}{3} = 32.6$
 $\frac{32.6}{3} \times 0.10 = 10.87$
 F, G, H

B, C, D
 $\frac{31.9 + 31.9 + 34.1}{3} = 32.6$
 $\frac{32.6}{3} \times 0.10 = 10.87$
 D, E, F
 $\frac{31.9 + 31.9 + 34.1}{3} = 32.6$
 $\frac{32.6}{3} \times 0.10 = 10.87$
 F, G, H
 $\frac{31.9 + 31.9 + 34.1}{3} = 32.6$
 $\frac{32.6}{3} \times 0.10 = 10.87$

- PERCOLATION TEST SHEET - Site 2

Test hole location 2340 FORST EAST LOT Hole # 24 Date test hole was prepared: 7/25/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches <u>0-6</u> <u>6-12</u>	soil texture: <u>Loam Topsoil</u> <u>CLAY</u>	soil color <u>Black</u> <u>Brown</u>
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Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 7/25/95 3pm Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: Automatic Siphon
 Percolation test conducted by: R. Koch Percolation test started at 1 (am/pm) 7/26/95
 Maximum water depth above hole bottom during test: 6 inches

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	CONVERSIONS
<u>104</u> <u>134</u>	START <u>30</u>	<u>6</u> <u>5 3/16</u>	<u>13/16</u>	<u>0.81</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ A}$	1/16 = .06 1/8 = .13 3/16 = .19
<u>137</u> <u>207</u>	REFILL <u>30</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ B}$	1/4 = .25 5/16 = .31
<u>210</u> <u>240</u>	REFILL <u>30</u>	<u>6</u> <u>5 1/4</u>	<u>3/4</u>	<u>0.75</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ C}$	3/8 = .38 7/16 = .44
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ D}$	1/2 = .5
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ E}$	9/16 = .56
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ F}$	5/8 = .63
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ G}$	11/16 = .69
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ H}$	3/4 = .75
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ I}$	13/16 = .81
---	REFILL	---	---	---	$\frac{\text{TIME}}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{\text{PERC}}{\text{PERC}} \text{ J}$	7/8 = .88 15/16 = .94

Ten Percent Calculation *

H, B, C
$\frac{\text{Largest } \# \text{ of ABC}}{\text{Smallest } \# \text{ of ABC}} = \frac{37}{3} = 12.33$
$\frac{\text{Largest } \# \text{ of ABC}}{\text{Smallest } \# \text{ of ABC}} \times 0.10 = 1.233$
C, D, E
$\frac{\text{Largest } \# \text{ of CDE}}{\text{Smallest } \# \text{ of CDE}} = \frac{37+40+40}{3} = 39$
$\frac{\text{Largest } \# \text{ of CDE}}{\text{Smallest } \# \text{ of CDE}} \times 0.10 = 3.9$
E, F, G
$\frac{\text{Largest } \# \text{ of EFG}}{\text{Smallest } \# \text{ of EFG}} = \frac{37+40+40}{3} = 39$
$\frac{\text{Largest } \# \text{ of EFG}}{\text{Smallest } \# \text{ of EFG}} \times 0.10 = 3.9$

B, C, D
$\frac{\text{Largest } \# \text{ of BCD}}{\text{Smallest } \# \text{ of BCD}} = \frac{40}{3} = 13.33$
$\frac{\text{Largest } \# \text{ of BCD}}{\text{Smallest } \# \text{ of BCD}} \times 0.10 = 1.333$
D, E, F
$\frac{\text{Largest } \# \text{ of DEF}}{\text{Smallest } \# \text{ of DEF}} = \frac{40+40+40}{3} = 40$
$\frac{\text{Largest } \# \text{ of DEF}}{\text{Smallest } \# \text{ of DEF}} \times 0.10 = 4.0$
F, G, H
$\frac{\text{Largest } \# \text{ of FGH}}{\text{Smallest } \# \text{ of FGH}} = \frac{40+40+40}{3} = 40$
$\frac{\text{Largest } \# \text{ of FGH}}{\text{Smallest } \# \text{ of FGH}} \times 0.10 = 4.0$

PERCOLATION TEST SHEET - Site 2

Test hole location 2340 FOST (East lot) Hole # 3A Date test hole was prepared: 7/25/95
 Depth of hole bottom: 12 inches Diameter of hole: 6 inches
 Soil Data from test hole:

depth, inches <u>0-18</u>	soil texture: <u>Loam Topsoil</u>	soil color <u>Black</u>
_____	_____	_____
_____	_____	_____
_____	_____	_____

Method of scratching sidewall: Nail Depth of pea size gravel in bottom of hole: 2 inches
 Date and hour of initial water filling: 7/25/95 3pm Depth of initial water filling: 12 above hole bottom
 Method used to maintain 12" of water depth in hole for 4 hours: Automatic Siphon
 Percolation test conducted by: R. Kahn Percolation test started at 1 (am/pm)
 Maximum water depth above hole bottom during test: 6 inches 7/26/95

TIME	INTERVAL (MINUTES)	WATER DEPTH	WATER DROP (fraction)	WATER DROP (decimal)	PERC RATE CALCULATION	CONVERSIONS
<u>108</u> <u>138</u>	START <u>30</u>	<u>6</u> <u>5 11/16</u>	<u>15/16</u>	<u>0.94</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{31.9}{\text{PERC}} \text{ A}$	1/16 = .06 1/8 = .13 3/16 = .19
<u>140</u> <u>210</u>	REFILL <u>30</u>	<u>6</u> <u>5 11/8</u>	<u>7/8</u>	<u>0.88</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{34.1}{\text{PERC}} \text{ B}$	1/4 = .25 5/16 = .31
<u>214</u> <u>244</u>	REFILL <u>30</u>	<u>6</u> <u>5 1/2</u>	<u>7/8</u>	<u>0.88</u>	$\frac{30}{\text{TIME}} \div \frac{\text{DROP}}{\text{(Decimal)}} = \frac{34.1}{\text{PERC}} \text{ C}$	3/8 = .38 7/16 = .44
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP}} = \text{PERC} \text{ D}$	1/2 = .5
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP}} = \text{PERC} \text{ E}$	9/16 = .56 5/8 = .63
_____	REFILL	_____	_____	<u>Min 1/16</u>	$\frac{\text{TIME}}{\text{DROP}} = \text{PERC} \text{ F}$	11/16 = .69
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP}} = \text{PERC} \text{ G}$	3/4 = .75
_____	REFILL	_____	_____	_____	$\frac{\text{TIME}}{\text{DROP}} = \text{PERC} \text{ H}$	13/16 = .81 7/8 = .88 15/16 = .94

Ten Percent Calculation *

A, B, C
$\frac{\text{Largest \# of ABC}}{\text{Smallest \# of ABC}} = \frac{31.9}{2.2}$
$\frac{31.9}{\text{Smallest \# of ABC}} \times 0.10 = \frac{31.9}{3.2}$
C, D, E
$\text{Ave} = \frac{31.9 + 34.1 + 34.1}{3} = 33.4$
$\frac{\text{Largest \# of CDE}}{\text{Smallest \# of CDE}} \times 0.10 = \frac{33.4}{3}$
D, E, F
$\frac{\text{Largest \# of DEF}}{\text{Smallest \# of DEF}} \times 0.10 = \frac{33.4}{3}$
E, F, G
$\frac{\text{Largest \# of EFG}}{\text{Smallest \# of EFG}} \times 0.10 = \frac{33.4}{3}$
F, G, H
$\frac{\text{Largest \# of FGH}}{\text{Smallest \# of FGH}} \times 0.10 = \frac{33.4}{3}$

B, C, D
$\frac{\text{Largest \# of BCD}}{\text{Smallest \# of BCD}} \times 0.10 = \frac{34.1}{3.2}$
D, E, F
$\frac{\text{Largest \# of DEF}}{\text{Smallest \# of DEF}} \times 0.10 = \frac{34.1}{3.2}$
E, F, G
$\frac{\text{Largest \# of EFG}}{\text{Smallest \# of EFG}} \times 0.10 = \frac{34.1}{3.2}$
F, G, H
$\frac{\text{Largest \# of FGH}}{\text{Smallest \# of FGH}} \times 0.10 = \frac{34.1}{3.2}$